Advantage of Using Free Network Adjustment Technique in the Crustal Movement Monitoring Geodetic Networks

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ABSTRACT

There are numerous adjustment techniques that deal with the adjustment of geodetic networks but the least squares adjustment is the most common one. During the network adjustment procedure two techniques can be used, the free network adjustment technique and the constrained network adjustment technique. In order to determine the optimum technique for adjusting the geodetic networks, which used for the geodynamical purposes, data from two different geodetic networks "Sinai geodetic network, Egypt, and HGN network, South Korea" had been examined. The used networks had a different configuration and located in different areas with different seismic activity. The results show that both techniques have a high accuracy and no remarkable differences in terms of RMS. On the contrary, the resulted coordinates shows that the constrained network adjustment technique not only cause a remarkable distortion in the station final coordinates but also if the fixed points that define the datum parameters are changed different solutions for the coordinates will be determined. This distortion affect not only in the determination of point displacement but also in the estimation of the deformation parameters, which play a significant role in the geodynamical interpretation of results. Comparing the results which obtained from both techniques with the widely known geodynamical models of the area reviles that the free network adjustment technique results are clearly match with these models, while those obtained from the constrained technique didn't match at all. By considering the results it seams to be that the free network adjustment technique is the optimum technique, which can be used for the geodetic network adjustment.

Key words: adjustment, geodetic networks, distortion, least squares adjustment

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요 약: 측지망 조정에는 여러 가지 기법이 있으나 가장 보편적인 것이 최소자승법이다. 지각 변위 연구 등 지구역학적 연구를 위해 GPS 자료를 처리할 때 일반적으로 두 가지 망조정 기법이 사용 되는데 그 중 하나는 자유망 조정법이고 나머지 하나는 제한적망 조정법이다. 이 두가지 기법 중 에서 어느 것이 더 지구역학적 목적에 적절한지를 결정하기 위해 두 개의 다른 측지망 즉, "이집 트의 시나이 측지망"과 "한국의 HGN 측지망"의 데이터를 사용하여 조사하였다. 사용된 두 개의 망은 서로 모양이 다를 뿐만 아니라 지진학적 활동이 다른 지역에 위치해 있다. 이 연구의 결과는 두 기법이 모두 RMS 입장에서 보면 고정밀성을 가지고 있고 큰 차이가 없음을 보인다. 그러나 좌 표값 결과를 볼 때 제한적망 조정법이 자유망 조정법에 비해 더 많은 왜곡을 일으켰을 뿐만 아니 라 측지망을 정의하는 고정점이 변하면 좌표값도 다르게 결정되었다. 이러한 왜곡은 측정점의 위 치 이동량은 물론 자료처리 결과의 지구역학적 해석에 중요한 역할을 하는 변위요소들의 추정에도 영향을 미친다. 이 연구의 대상 지역에 대해 널리 알려진 지구역학 모델과 두 기법으로부터 얻어 진 결과를 비교해 보면, 자유망 조정법의 결과가 이들 모델에 확실히 잘 맞는 반면 제한적망 조정 법의 결과는 전혀 맞지 않는 것으로 나타났다. 이런 결과를 종합해 볼 때 자유망 조정법이 지구역 학적 목적에 더 부합하는 측지망 조정기법이라고 판단된다. 주요어: 조정, 측지망, 왜곡, 최소자승법

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1. Introduction

Numerous adjustment techniques can be used for geodetic network adjustment, but the least squares adjustment is the most common technique which widely used in geodetic network adjustment (Vaniček and Krakiwsky, 1982). The adjustment of a GPS geodetic network data may be done by one of the following methods, free network adjustment technique or constrained network adjustment technique (Nassar, 1985 and 1986; Bock, 1982 and 1983; Strang and Borre, 1992). The term free network refers to a network whose adjustment is made free from any kind of external constrains. This means that the network can freely translate, rotate or undergo scale change in space. Based on that concept all the network positions will receive some corrections after the adjustment.

While, the basic of constrained network adjustment is to choose some network stations to be fixed or their coordinates are assigned known variances. If the fixed points that define the datum parameters are changed different solutions for the coordinates will be determined (Mousa, 1992).

2. GPS geodetic networks and campaigns

The data, which used in our study, were collected from two different geodetic networks "Sinai geodetic network, Egypt and HGN, South Korea", table (1) represents the campaign's information for the both geodetic networks. Sinai geodetic network, which composed of ten geodetic stations, is mainly constructed to monitor the recent crustal movements in and around the Sinai Peninsula (Hamdy, 2001). Sinai Peninsula is considered to be a seismo active area where it is surrounded by three seismic zones (Fig. 1a), the largest earthquake which occurred in this area took place in Nuweiba city at November 22, 1995, and it has a magnitude of Mw = 7.1(Kebeasy, 1990; Said, 1990; Tealeb, 1995; Kimata et al., 1996; Atiya, 1997; Hamdy, 2001). While, the Honam Geodetic Network " HGN" is apart of KAO geodetic network "Korea Astronomy Observatory" and it is composed of five geodetic stations (Hamdy et al., 2002). Generally the Korean peninsula is considered to be having a relatively low seismic activity with respect to the seismic activity in Sinai Peninsula (Figs. 1a and 1b) (KMA, 2001; Hamdy et al., 2002). Through out the previous information it is easy to find out that the two networks are differing from each other not only in their location "latitude and longitude" and their configuration but also they differ from each other by the amount of expected movements.

3. GPS data Analysis

The observed data from both geodetic networks

| Table 1. Networks Campaign S miorman | Table | 1. | 1. Networks | Campaign's | Information |
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| Network | Campaigns | Campaigns length | Remarks |
|-----------------|-----------|------------------|--|
| Circi Netrovela | 1994 | 7.1 | In 1995 an earthquake with a magnitude 7.1 |
| Sinai Network | 1996 | /days | take place near by Nuweiba city. |
| UCN Network | 2001 | 5.1 | |
| HUN NELWORK | 2002 | Sdays | |



Fig. 1. (a) Seismic map of Sinai Peninsula, (b) Seismic map of the Korean Peninsula.

have been analyzed by using Bernese software version 4.2 (Beutler *et al.*, 2001). The IGS final orbit "Prices ephemeris" had been used in the orbit determination and the quasi- ionosphere free (QIF) strategy was used for the ambiguity resolution. We estimated the baselines with L3 ionosphere free linear combination. In adjustment step for both geodetic network two-adjustment techniques "free network adjustment technique and constrained network adjustment technique" were used in order to compare the results of both techniques.

Regarding to table (1) the considered epochs for Sinai geodetic networks are 1994 and 1996 respectively. During the adjustment step of Sinai geodetic network by using free network adjustment technique the network C.g. "Center of gravity" is considered to be fixed, resulting from that all the network stations coordinates receive some corrections after the adjustment step "from the definition". The resulted coordinates together with their RMS "root mean squares error" are represented in table (2). In case of constrained network adjustment technique "if the station which defined the datum is changed different solutions will be obtained, from the definition" two strategies have been used in order to investigate the stability of this solution. Firstly, station HUR "this station have a moderate longitude with respect to other network stations and lying in the southern part of the network (Fig. 2a)" is considered to be fixed during the network adjustment. Secondly, we fix FAN station, which is considered to be lying in the northwest part of Sinai geodetic network (Fig. 2a). The results, which obtained from the constrained network adjustment technique, are represented in table (2). In order to distinguish the distortion occurred in the network final coordinates figure (3) has been constructed to illustrate the relation shape between the initial coordinates "prior coordinates" and the final coordinates "adjusted coordinates". From figure 3, it is clear that" in case of HUR fixed" all the network stations move to the south direction to ward station HUR. While in the east-west direction some stations move to the east while others move to the west depending on it is location with respect to station HUR. While the resulted distortion from the fixation of FAN station is so clear where all the network stations move toward the fixed station "FAN station" i.e. all the network



Fig. 2. (a) Sinai geodetic network, (b) HGN geodetic network.



▲ HUR

Fig. 3. Distortion in Sinai geodetic network coordinates resulting from the constrained network adjustment technique.

station move to the northwest direction.

In case of HGN geodetic network we also use both adjustment techniques "free network adjustment technique and constrained network adjustment technique" in the network adjustment step. The free network adjustment technique had been applied to the HGN network data and the resulted final coordinates and their RMS are represented in table (3). In constrained network adjustment technique we also use two different solution strategies "by using two different fixed stations". In the first solution strategy we fix station DAEJ while in the second



Fig. 4. Distortion in HGN geodetic network coordinates resulting from the constrained network adjustment technique.

strategy we fix station BHAO (Table 3 and Fig.2b). Moreover the changes that occurred in the network stations final coordinates, which resulted from constrained network adjustment technique, are represented in figure (4). In case of fixing DAEJ station most of the network stations move toward it but this movement is not so clear as that in case of HUR and FAN stations in Sinai geodetic network "maybe because DAEJ station is near by the HGN C.g.". In case of BHAO station all the network stations move to the south and to the west directions, which is clear from figure (4), i.e. all the network stations move towards BHAO station.

4. Discussion

The obtained results from Sinai geodetic network computation reveals that the RMS in cases of free network adjustment technique is less than 1.2mm while in case of constrained network adjustment technique the resulted RMS are not greater than 1.7mm (Table 2). For HGN geodetic network and through out table 3 it is clear that the RMS in case of free network adjustment technique are less than 0.6mm and in case of constrained network adjustment technique it is less than 1.0 mm. Generally, by examining the RMS of both techniques the RMS that resulted from the free network adjustment technique shows a slightly higher accuracy (tables 2 and 3).

Since the displacement is considered to be the only required input data when we are calculating the deformation tensor (Grant, 1990), we also examine the results to investigate the effect of different solution on the resulted displacements. The results, which obtained from Sinai geodetic network was used to calculate the displacement in Sinai peninsula by considering epoch 1994 as a reference epoch for 1996. Firstly the displacement was calculated using the coordinates which resulted from the free network adjustment technique (Fig. 5a and Table4). Secondly the displacement was calculated using the resulted coordinates which obtained from constrained network adjustment technique (Figs. 5b and 5c and Table4). Through out these figures and table it is recognized that there is a remarkable change in the displacement components "dn and de" not only between the free network adjustment technique and the constrained network adjustment technique but also when we use different strategies in constrained network adjustment technique different displacement components are obtained. In case of HGN geodetic network displacement computation we use the epoch 2001 as a reference epoch for the epoch 2002 and by comparing the results, it is found that there is a remarkable differences not only in the station displacement components but also in their corresponding azimuth (Figs.6a, 6b, 6c and Table 5). Finally, we compare the obtained displacement

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| | | | | | | * |

| Year | | | 2001 | | | | | | 2002 | | | |
|-------------------|-------------------|-----------|------------------|-----------|-------------------------|--------------|-----------------|------------|-----------------|--------|--------------|--------|
| Station | Х | RMS X | Υ | RMS Y | Ζ | RMS Z | Х | RMS X | Υ | RMS Y | Ζ | RMS Z |
| SKMA [*] | -3043536.8299 | 0.0004 | 4050958.9427 | 0.0003 | 3861031.6215 | 0.0006 | -3043536.8300 | 0.0003 | 4050958.9425 | 0.0004 | 3861031.6201 | 0.0004 |
| SKCH [*] | -3126511.1997 | 0.0004 | 3921458.1462 | 0.0004 | 3927386.0419 | 0.0005 | -3126511.1994 | 0.0004 | 3921458.1438 | 0.0005 | 3927386.0421 | 0.0005 |
| $BHAO^*$ | -3243254.5340 | 0.0005 | 4008470.7323 | 0.0004 | 3743543.6834 | 0.0005 | -3243254.5327 | 0.0005 | 4008470.7347 | 0.0006 | 3743543.6871 | 0.0005 |
| MKPO* | -3109396.4416 | 0.0005 | 4220348.6338 | 0.0006 | 3621241.1409 | 0.0004 | -3109396.4430 | 0.0003 | 4220348.6361 | 0.0005 | 3621241.1394 | 0.0004 |
| $DAEJ^*$ | -3120042.2237 | 0.0004 | 4084614.8118 | 0.0003 | 3764026.7873 | 0.0006 | -3120042.2238 | 0.0003 | 4084614.8095 | 0.0004 | 3764026.7862 | 0.0004 |
| SKMA [#] | -3043536.8297 | 0.0006 | 4050958.9426 | 0.0008 | 3861031.6216 | 0.0007 | -3043536.8298 | 0.0005 | 4050958.9448 | 0.0007 | 3861031.6210 | 0.0006 |
| SKCH [#] | -3126511.1998 | 0.0004 | 3921458.1460 | 0.0006 | 3927386.0420 | 0.0006 | -3126511.1993 | 0.0005 | 3921458.1461 | 0.0007 | 3927386.0430 | 0.0007 |
| $BHAO^{*}$ | -3243254.5346 | 0.0006 | 4008470.7329 | 0.0007 | 3743543.6842 | 0.0007 | -3243254.5326 | 0.0006 | 4008470.7370 | 0.0008 | 3743543.6880 | 0.0007 |
| MKPO [#] | -3109396.4416 | 0.0004 | 4220348.6339 | 0.0006 | 3621241.1407 | 0.0005 | -3109396.4428 | 0.0005 | 4220348.6383 | 0.0007 | 3621241.1400 | 0.0006 |
| $DAEJ^{\#}$ | -3120042.2237 | 0.0000 | 4084614.8118 | 0.0000 | 3764026.7873 | 0.0000 | -3120042.2237 | 0.0000 | 4084614.8118 | 0.0000 | 3764026.7870 | 0.0000 |
| SKMA | -3043536.8290 | 0.0008 | 4050958.9418 | 0.0010 | 3861031.6209 | 0.0009 | -3043536.8311 | 0.0007 | 4050958.9399 | 0.0008 | 3861031.6163 | 0.0008 |
| SKCH | -3126511.1991 | 0.0007 | 3921458.1449 | 0.0009 | 3927386.0408 | 0.0008 | -3126511.2005 | 0.0007 | 3921458.1413 | 0.0009 | 3927386.0385 | 0.000 |
| BHAO | -3243254.5340 | 0.0000 | 4008470.7323 | 0.0000 | 3743543.6834 | 0.0000 | -3243254.5340 | 0.0000 | 4008470.7323 | 0.0000 | 3743543.6834 | 0.0000 |
| MKPO | -3109396.4409 | 0.0007 | 4220348.6331 | 0.0009 | 3621241.1404 | 0.0008 | -3109396.4441 | 0.0007 | 4220348.6335 | 0.0009 | 3621241.1355 | 0.0008 |
| DAEJ | -3120042.2231 | 0.0007 | 4084614.8111 | 0.0008 | 3764026.7868 | 0.0008 | -3120042.2249 | 0.0007 | 4084614.8070 | 0.0008 | 3764026.7824 | 0.0007 |
| RMS is | represented in me | ter. * Fr | ee network solut | ion. # DA | VEJ station cons | idered to be | e fixed. ^ BHAO | station co | onsidered to be | fixed. | | |

Table 3. The Stations final coordinates in WGS 84 reference frame system, resulting from using different adjustment techniques for HGN network.





Fig. 5. Displacement in Sinai geodetic network.



c. Constrained network solution (BHAO is fixed)

Fig. 6. Displacement in HGN geodetic network.

| | | | | | | Solutio | on Type | | | | | | | |
|---------|---------|---------------------------|---------|--------|---------|---------------------------------|-----------|--------|---------|-----------|----------|--------|--|--|
| Station | | Ence M | t mode | | | Constrained net work adjustment | | | | | | | | |
| Station | | FIEE IN | et work | | | Station H | IUR fixed | 1 | S | Station F | AN Fixed | | | |
| | d_n | \mathbf{d}_{e} | azimuth | Dis. | d_n | d_{e} | azimuth | Dis. | dn | d_e | azimuth | Dis. | | |
| HUR | 0.0039 | -0.0248 | 30 | 0.0251 | 0 | 0 | 0 | 0 | 0.0347 | 0.0424 | 70 | 0.0548 | | |
| GAR | 0.0008 | -0.0053 | 30 | 0.0054 | -0.018 | -0.018 | 230 | 0.0228 | 0.0214 | -0.0339 | 332 | 0.0401 | | |
| FAN | -0.0071 | 0.0352 | 140 | 0.036 | 0.0515 | 0.0515 | 170 | 0.0549 | 0 | 0 | 0 | 0 | | |
| RAH | -0.0552 | -0.0024 | 110 | 0.0552 | -0.0121 | -0.0121 | 140 | 0.0612 | -0.0244 | 0.0146 | 78 | 0.0285 | | |
| ZEN | -0.0053 | 0.0359 | 8 | 0.0362 | 0.0122 | 0.0122 | 339 | 0.0127 | -0.0003 | -0.0668 | 344 | 0.0668 | | |
| KTH | -0.0003 | -0.0151 | 300 | 0.0151 | 0.025 | 0.025 | 232 | 0.0281 | -0.0401 | -0.0337 | 318 | 0.0524 | | |
| TUR | -0.0127 | 0.0119 | 260 | 0.0174 | -0.0307 | -0.0307 | 230 | 0.039 | -0.0395 | -0.0332 | 318 | 0.0516 | | |
| MOH | 0.0165 | 0.0253 | 334 | 0.0302 | 0.0287 | 0.0287 | 264 | 0.0288 | -0.069 | -0.0092 | 330 | 0.0697 | | |
| DAH | -0.0916 | 0.0857 | 260 | 0.1254 | -0.0147 | -0.0147 | 253 | 0.1444 | 0.0164 | -0.1475 | 275 | 0.1484 | | |
| NKL | 0.0349 | -0.068 | 112 | 0.0764 | -0.0162 | -0.0162 | 140 | 0.0821 | 0.039 | -0.0229 | 100 | 0.0452 | | |

Table 4. Displacement in Sinai geodetic network resulted from different adjustment technique.

Azimuth measured clockwise and the displacements are in meter and dn and de are the displacements components.

Table 5. Displacement in HGN geodetic network resulted from different adjustment technique.

| | | Solution Type | | | | | | | | | | | |
|---------|---------|---------------|---------|----------|----------|---------|---------|-----------|----------|----------|---------|---------|--|
| Station | | E N. | 41- | | | | Constr | ained net | work ad | justment | | | |
| Station | | Free Ne | S | tation D | AEJ fixe | d | S | tation BH | IAO Fixe | d | | | |
| | d_n | de | azimuth | Dis. | d_n | de | azimuth | Dis. | dn | de | azimuth | Dis. | |
| SKMA | 0.00018 | 9.4E-05 | 55 | 0.0002 | -0.0011 | 0.0019 | 121 | 0.0022 | 0.0027 | -0.0005 | 349 | 0.00275 | |
| SKCH | -0.0021 | 0.0012 | 32 | 0.0024 | -4E-05 | -0.0003 | 210 | 0.00025 | 0.00304 | -0.0023 | 322 | 0.00381 | |
| BHAO | -0.0004 | -0.0027 | 235 | 0.0027 | -0.0037 | 0.00255 | 146 | 0.00449 | 0 | 0 | 0 | 0 | |
| MKPO | -0.0006 | 0.00252 | 182 | 0.0026 | 0 | 0.0045 | 97 | 0.0045 | 0.0026 | 0.00189 | 36 | 0.00321 | |
| DAEJ | -0.0007 | 0.00217 | 25 | 0.0023 | 0 | 0 | 0 | 0 | 0.0036 | -0.0026 | 324 | 0.00444 | |

Azimuth measured clockwise and the displacements are in M and dn and de are the displacements components.

for both geodetic networks with the widely known geodynamical models for both areas e.g., (Badawy and Horvath, 1999; Hamdy 2001 "for Sinai", Chough *et al.*, 2000; Hamdy *et al.*, 2002 "for HGN") and it is found that the results obtained from free net work adjustment technique is concede with these models.

5. Conclusions

The previous discussion reveals that both geodetic network adjustment techniques give high accuracy but the free network adjustment technique shows slightly high accuracy than the constrained network adjustment technique. The distortion occurred in the network station coordinates resulting from using the constrained network adjustment technique affect not only the displacement calculation but also in its azimuth, which affect on the deformation parameters estimation. Comparing the resulted displacement from both techniques with the widely known geodynamical models of both areas show that the free network adjustment result is totally concede with these models. The free network adjustment technique is the optimum adjustment technique, which can be used in the adjustment of the geodetic network for the geodynamical applications.

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